

INVESTIGATION ON THE BEHAVIOR OF RIGID POLYVINYLCHLORIDE PIPES SUBJECTED TO UNIAXIAL COMPRESSION LOADS

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ABSTRACT

The objective of this research was to investigate the capability of rigid Polyvinylchloride (PVC-U) pipes to sustain axial loads. The behavior of PVC-U pipes specimens subjected to short-term uniaxial compression loads were experimentally investigated. Results of the load-displacement tests on pipes of different wall thickness, diameter, and specimen heights were recorded. The experimental test results show that the PVC-U pipes are capable of supporting loads greater than the required design load (3.4 kN). Increasing the pipe specimen height decreases the pipe strength for supporting the axial loads. Euler buckling was observed in the pipe specimens with small diameter and long axial height. Furthermore, an attempt has been made to utilize the ANSYS finite element commercial package to model the pipe system using eight noded brick element and the results were compared with those obtained from the experimental tests. Good agreement was found between the two sets of results, within the elastic range of the load-displacement curve.

Keywords: *PVC-U pipes, polyvinylchloride, uniaxial load, models*

1. INTRODUCTION

The long-term behavior of various Polyvinylchloride (PVC) products application has a good record of long-term durability [1, 2]. The range of PVC's application demonstrates that this is the most versatile among thermoplastics [3]. The good performance and low cost of polyvinylchloride products make this polymer very suitable for applications in buildings, mainly in exterior applications, like sheet piles, window profiles, cladding structures and siding [1, 4].

The phenomenon of cylindrical tubes made from different materials (steel, aluminum, and PVC-U) and various geometrical shapes collapsing in axial compression is considered one of the most efficient means of crash energy absorption, and therefore their collapse behavior has been studied by many researchers [5 - 23].

The collapsing of PVC-U tubes under axial compression loads were investigated by some researchers for the purpose of crash energy absorption devices. For instance, Mamalis *et al.* 1986 [5] reported the crumpling of thin-walled PVC-U tubes when subject to axial load. They examined the tubes with external circumferential grooves. They reported non-axisymmetric

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diamond mode of deformation. Later, Mamalis *et al.* 1986 [6] presented collapse mechanisms for the axial crumpling of thin-walled circular cones and truncated circular cones. Their experiments were carried out on shells made of PVC-U; their modes of failure and load-deflection characteristics were determined. Mamalis *et al.* 1989 [7] also investigated theoretically and experimentally the axial collapse of thin-walled square tubes and frusta of PVC-U. The deformation modes and load-compression characteristics were reported and deformation mechanisms were proposed to describe the crumpling process of the shells. Singace 2000 [13] studied analytically and experimentally the collapse of axially crushed sawdust-filled PVC-U tubes. It was found that, empty PVC-U tubes normally collapse into multi-lobe mode when axially crushed but at a certain value of wood filler density, the collapse reverts from multi-lobe to concertina mode and therefore the overall energy absorption performance of the PVC-U tubes is enhanced.

In the present study, a commercial PVC-U pressure pipes available in the local market [24] were tested under a short-term uniaxial compression loads to investigate the strength of the pipes under axial compression loads. The main purpose of the test is to evaluate the capability of the pipe to be used as a subsurface water storage system, which is required to support an axial load of 3.4 kN, calculated based on the manuals of American Society of Civil Engineers 1992 [24]. The effect of different pipe wall thickness, diameter, and height on the pipe strength was investigated.

2. MATERIALS AND METHODS

2.1 Materials and specimen preparation

The thickness of the PVC-U pipes walls for pipe sizes chosen for the analysis are presented in Table 1. The heights of the pipe specimens for each diameter and thickness are 100 mm, 200 mm, 300 mm, 400 mm, 500 mm and 600 mm. Thus, total of 48-specimens of PVC-U pipes were tested in uniaxial compression. Some of the specimens prepared are shown in Fig. 1. The pipe specimen material was PVC-U. Details related to the material properties were provided by the manufacturer of the pipes [25] and are shown in Table 2.

Table 1: Dimension and thickness of PVC-U pipes

External pipe diameter (D) (mm)	Pipe wall thickness (t) (mm)
89.1	3.2
	3.8
	5.0
	6.1
	6.1
114.3	4.0
	4.7
	6.4
	9.5

2.2 Test procedure

Each specimen was placed between parallel platens of an Instron 8500 digital-testing universal machine with a full-scale load range of 250 kN (Fig. 2). The axial load was applied at a crosshead speed of 2 mm/min until the specimen yield point was reached. The load-

displacement curve for each specimen was recorded by an automatic data acquisition system. The temperature and the humidity during the tests were maintained 23°C and 50% respectively.

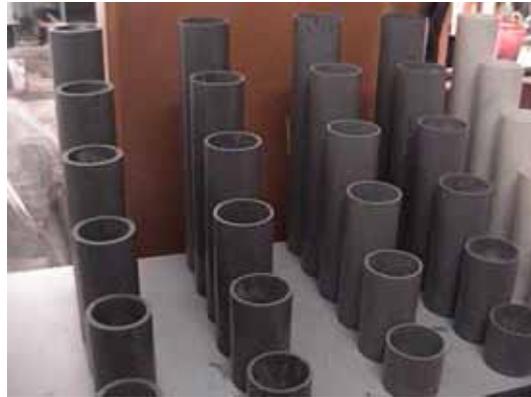


Fig. 1: Pipes specimens prepared for test

Table 2: Properties of PVC-U pipes

Property	Unit	Value
Specific Gravity	-	1.42
Hardness	Rockwell R	120
Water Absorption	mg/cm ²	0.05
Tensile Strength	MPa	45
Elongation at Break	%	80
Compressive Strength	MPa	66
Modulus of Elasticity	MPa	2700
Izod Impact Strength	kJ/m ²	6
Poisson's Ratio	-	0.37



Fig. 2: Placement of a specimen on the testing machine

3. CLASSICAL FORMULAS FOR STRESS AND STRAIN

Formulas for membrane stresses and deformations in thin-walled pressure vessels were used for predicting the stress and the displacement (deformation) of the pipes when subjected to an axial compression load. Young and Budyanas 2002 [26] presented many formulas for thin-walled pressure vessels for different cases to determine the membrane stresses and displacements. In this study the closest case may be applicable to the pipe test is shown in appendix A (Fig. A.1).

In the theoretical analysis the axial load of 3.4 kN was applied on all the pipe specimens and the stresses, vertical displacements, and the radial displacements given by Equations (A.1), (A.2), and (A.3) in appendix A are calculated.

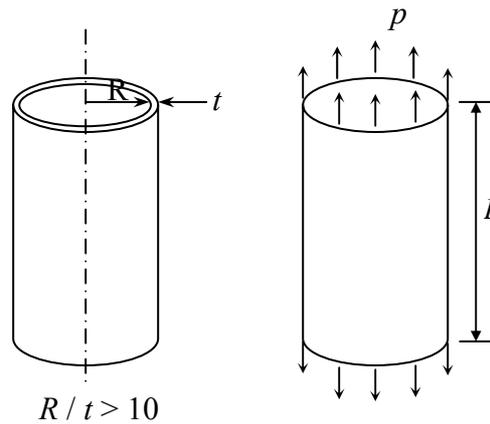


Fig. A.1: Cylindrical vessel under uniform axial load

4. FINITE ELEMENT MODELING

Finite Element Analysis (FEA) technique was used to determine the stresses and displacements for the pipes specimens. This technique was used due to the following reasons,

1. Equation (A.1) does not give the effect of the boundary condition at the pipe bottom on the stress distribution along the pipe.
2. To investigate the applicability of these equations for the pipe tests subjected to an axial compression load, as the equations are applicable for a cylindrical vessel subjected to an axial tension load.
3. To verify the effect of the ratio R/t on the results because the equations can be used for the ratio of R/t greater than 10 where some pipe specimens have ratio below this value.

Axial crushing of the PVC pipes was simulated using ANSYS Release 9.0 finite element code. In the analysis the model was fixed at one end (at the bottom) and uniformly distributed load was applied at the other end to simulate the testing condition (Fig. 3).

The pipe was modeled using Solid 3-D brick element available in the element library of the ANSYS commercial package called SOLID45 [27]. The entire pipe geometry is modeled with a relatively fine mesh to avoid distorted aspect ratios. A typical mesh is shown in Fig. 3.

The pipe was analyzed under different boundary conditions to investigate the condition that is close to the experimental tests of the pipe specimens, namely:

- (a) All the bottom nodes are restrained against z-direction movement.

- (b) All the bottom nodes are restrained against the translation in x , y , and z direction respectively.

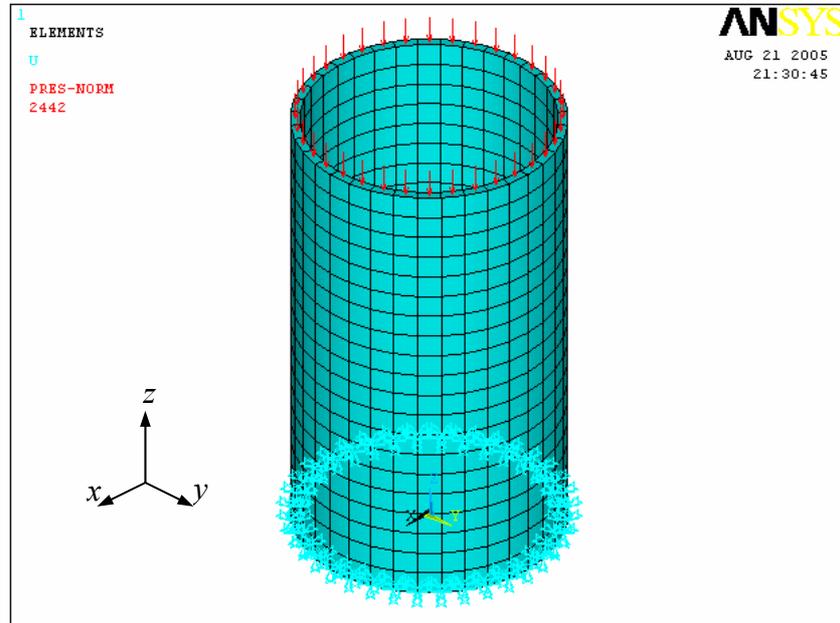


Fig. 3: Finite element modeling of pipe specimens

In the evaluation of the boundary condition, a pipe with 89.1 mm diameter, 3.2 mm wall thickness and 300 mm height is fixed first at the pipe bottom in the axial direction (z direction). The pipe was subjected to an axial compression load of 3.4 kN (Figs. 4(a) and (b)). It was found that,

1. The stress distribution in z direction σ_z (maximum stress) along the pipe height is constant and no stress concentration at any point of the pipe (Fig. 4(c)).
2. There is a horizontal displacement in one direction of the pipe (Fig. 4(d)).

In the next test, the same pipe was tested again but the pipe bottom was fixed in all directions (x , y and z) as shown in Fig. 5(a). It was found that,

1. The stress distribution σ_z is not constant along the pipe height and a maximum stress (stress concentration) developed at the bottom of the pipe. This indicates that the pipe failure may occur at this location. In addition, the stress at the pipe external wall is greater than the pipe internal wall stress at the pipe bottom, which means that a local buckling failure may be expected (Figs. 5(b) and (c)).
2. A very small horizontal displacement (0.026 mm) in a radial direction of the pipe is developed (Fig. 5(d)).

From the experimental tests on the pipes specimens, it was found that, most of the specimens failed at the bottom due to local buckling, and no horizontal displacement at one direction was observed on the pipe specimens. This illustrates that, the second case (b) (pipe bottom fixed in all directions) is closer to the experimental tests.

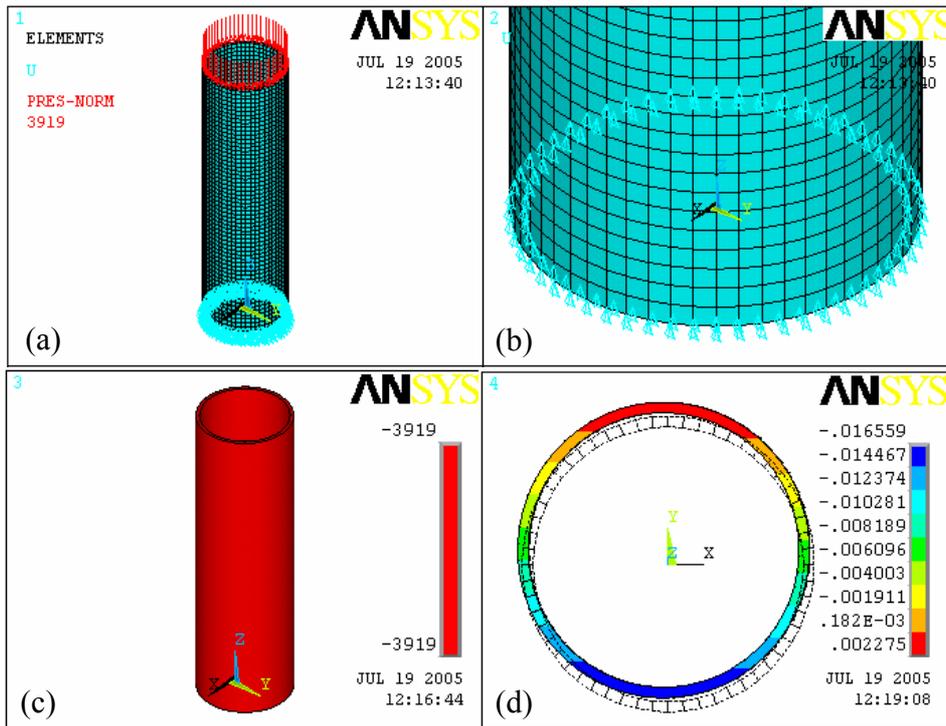


Fig. 4: Pipe fixed at the bottom in a vertical direction only, z

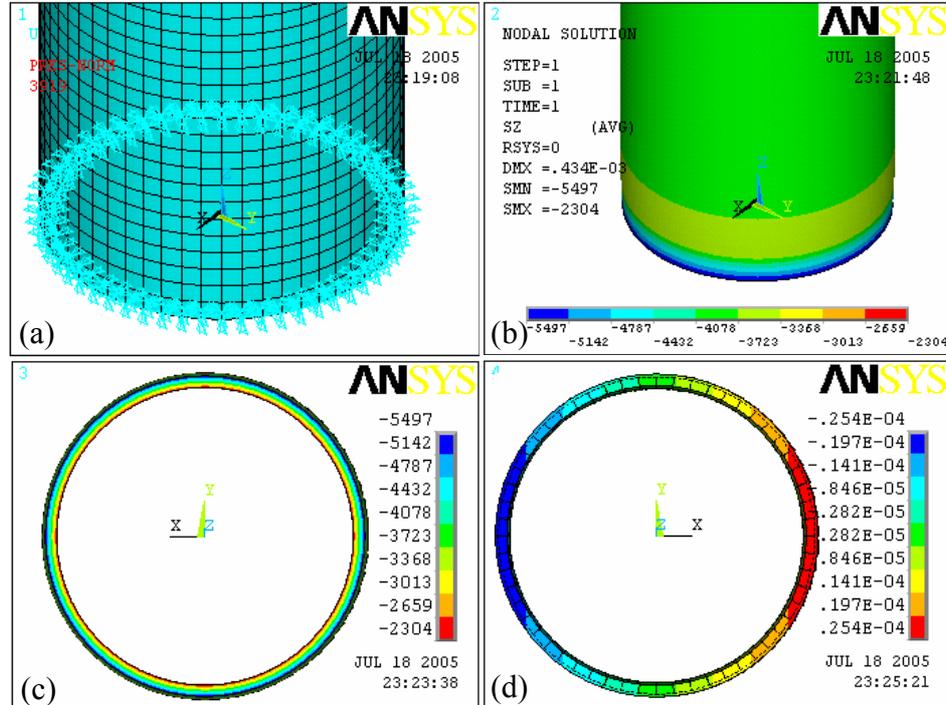


Fig. 5: Pipe fixed at the bottom in all directions

5. RESULTS AND DISCUSSION

5.1 Experimental results

The behavior of the PVC-U pipes under uniaxial compression loads with different thicknesses, diameters, and heights are presented in Figs. 6, 7, 8, 9, and 10.

5.1.1 Pipe of 89.1 mm diameter

Figure 6 shows that, the maximum compression load of the pipe decreases by increasing the pipe height to an average value of 13% when the pipe height is increased from 100 mm to 600 mm and a maximum reduction value of 28% occur with pipe thickness of 3.2 mm.

Load-displacement relationship for some selected specimens (3.2 mm pipe thickness) is presented in Fig. 7. The figure shows the effect of the pipe height in this relation. Euler buckling was observed with long pipe specimens (500 mm and 600 mm) as shown in Fig. 8.

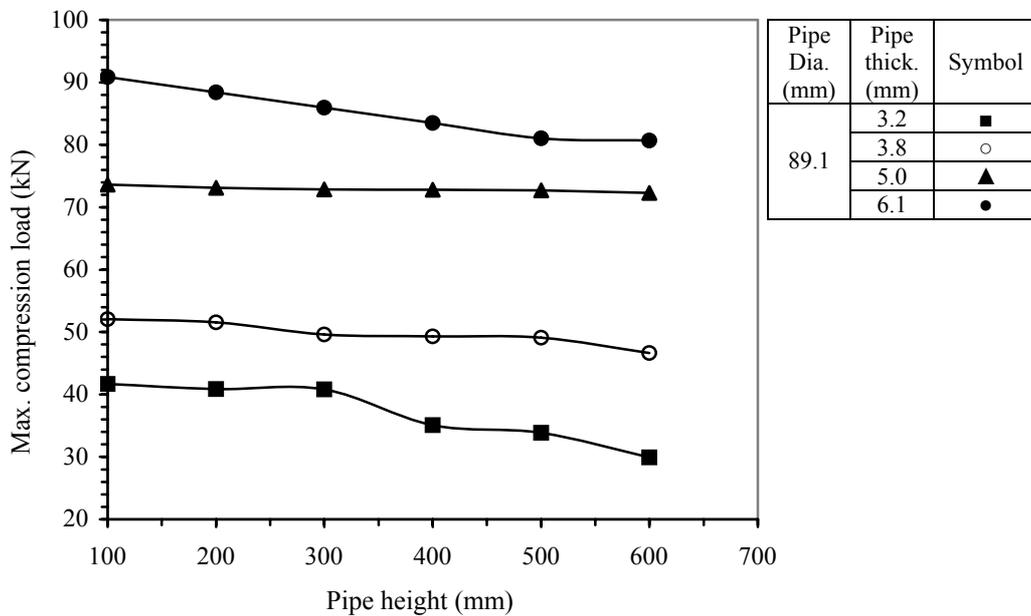


Fig. 6: Effect of the pipe height and thickness on the maximum compression load

5.1.2 Pipe of 114.3 mm diameter

Reduction of maximum pipe compression loads due to increasing pipe height was also observed in this pipe diameter but with lower observed values from the pipe of 89.1 mm diameter. The average value of 9% of the reduction in the pipe compression loads when the pipe height is increased from 100 mm to 600 mm and a maximum reduction value of 11% occur with pipe thickness of 6.4 mm (Fig. 9).

The effect of the pipe thickness and height on the load-displacement relation is presented in Fig. 10. No Euler buckling was observed for all pipe specimens heights for this pipe diameter.

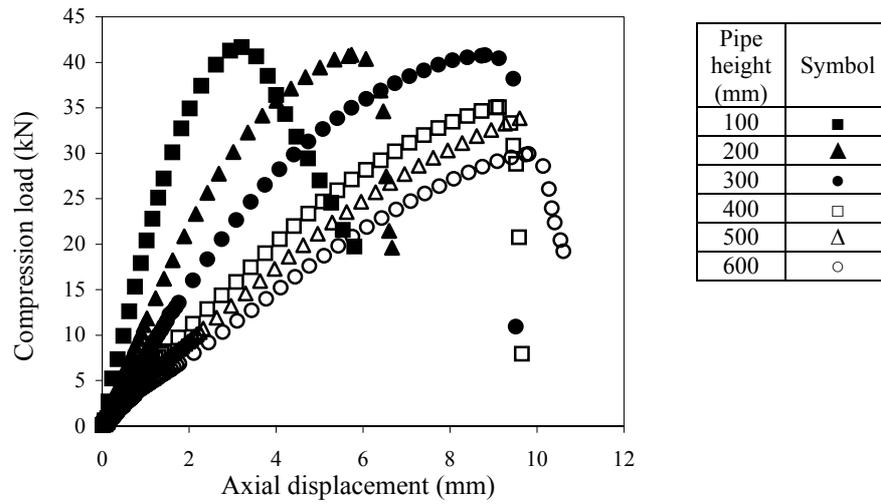


Fig. 7: Load-displacement relationship for the pipe with 89.1 mm diameter



Fig. 8: Buckling occurring with long pipe specimens

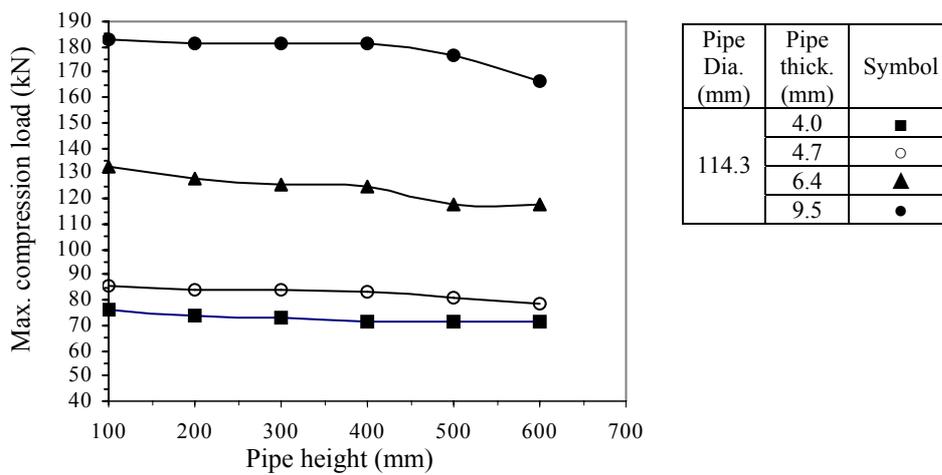


Fig. 9: Effect of the pipe height and thickness on the maximum compression load

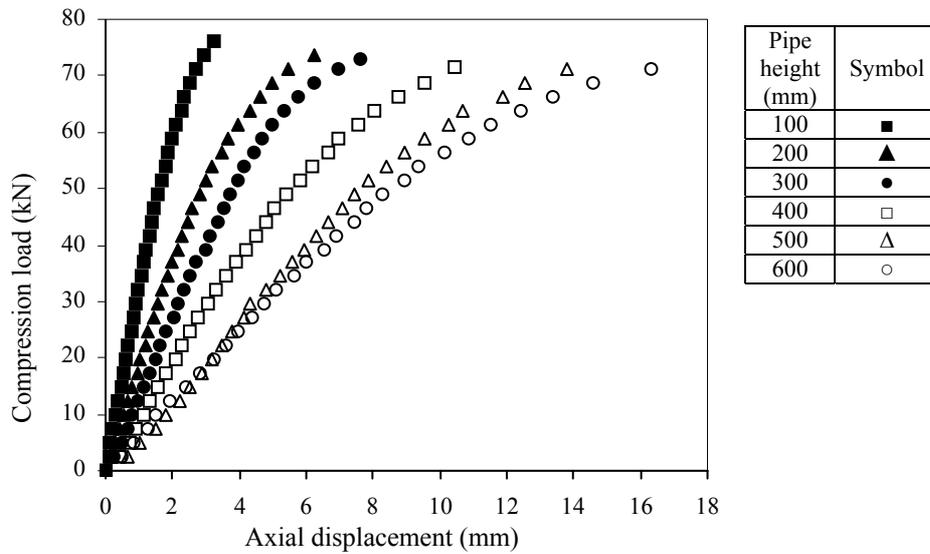


Fig. 10: Load-displacement relation of the pipe with 114.3 mm diameter

5.2 Theoretical results

5.2.1 Classical formula and FEA

Results of computed on stresses, vertical and radial displacements of the pipe specimens due to an axial compression load of 3.4 kN using Equations (A.1), (A.2), and (A.3) and the FEA are shown in the appendix A (Tables A.1 and A.2). The average and the maximum absolute differences of the axial displacements of all the pipe specimens by using Equation (A.2) and the FEA were 0.91% and 4%, respectively.

These results prove that, Equation (A.2) is applicable to predict the vertical displacements of the pipe specimens subjected to an axial compression load even if the ratio of R/t below the value of 10. But the average and the maximum value of the absolute differences of the radial displacements of all the pipe specimens by using Equation (A.3) and the FEA were 15.75% and 23.61% respectively. This may be due the effect of the pipe constraint, state of load (compression), and the ratio of R/t of some specimens is less than 10.

On the other hand, Equation (A.1) underestimates the values of the maximum stress along the pipe and does not give any idea for the stress distribution along the pipe due to the effect of the constraint on the pipe bottom. FEA demonstrates that, the stress concentration will develop at the pipe bottom and with values greater than that of Equation (A.1).

5.2.2 Pipes behavior using FEA

The behavior of the pipes under an axial uniform compression load is investigated by using the FEA techniques. A case of a pipe specimen (114.3 mm external diameter, 4 mm wall thickness and 300 mm height) was selected and subjected to an axial compression load of 3.4 kN. The stresses and displacements were determined and illustrated by the Figs. 11, 14, and 15.

Figure 11 shows that, the vertical stress distribution σ_z is approximately constant along the pipe height (approximately 2306 kN/m²) but a maximum stress of 3532 kN/m² developed at the external wall of the pipe bottom and a minimum stress of 1326 kN/m² at the internal wall of

pipe bottom. Due to the stress concentration around the external wall of the pipe bottom, the failure of the pipe is expected to occur at this location as local buckling. This result was verified by the experimental tests of the pipe specimens and was found that most of specimens failed at the pipe bottom as local buckling (Figs. 12 and 13).

The values of the stress at the radial direction (σ_x or σ_y) is approximately constant (approximately 169.459 kN/m²) along the pipe height and is also maximum on the external wall of the bottom of the pipe with a value of 1890 kN/m², which is lower than σ_z (Fig. 14).

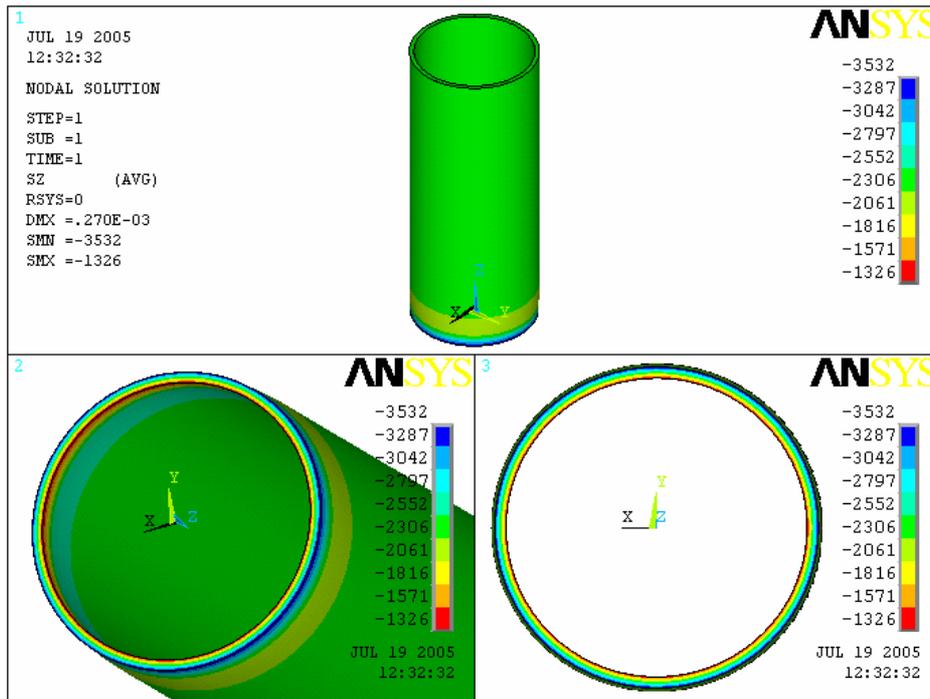


Fig. 11: Vertical stress distribution along the pipe wall height, σ_z

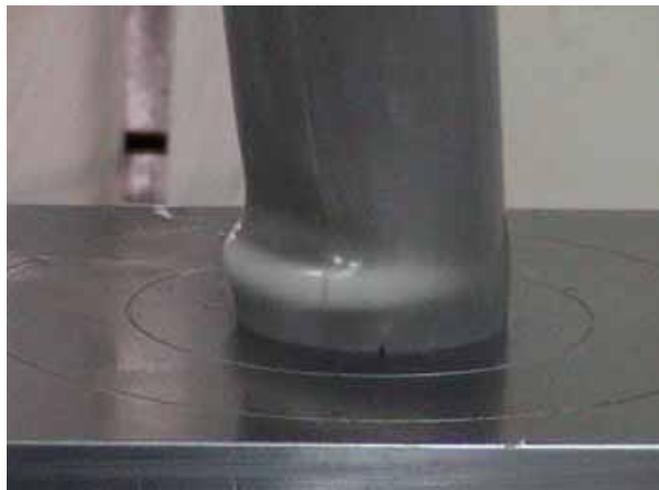


Fig. 12: Pipe specimen failed at the bottom



Fig. 13: Most of the pipe specimens failed at the bottom as local buckling

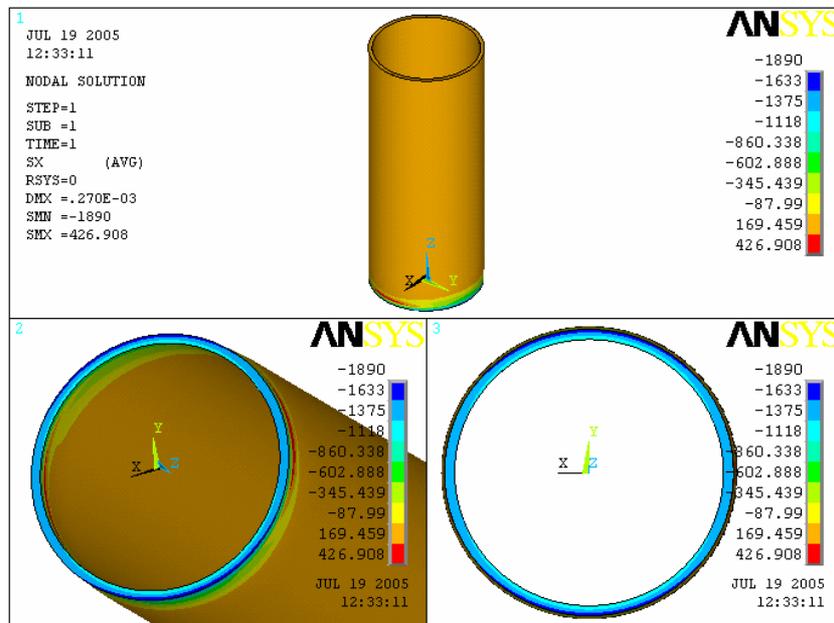


Fig. 14: Radial stress distribution along the pipe wall height, σ_x

The axial displacements ΔL of the pipe wall is shown in Fig. 15(a). Figure 15(b) and Fig. 15(c) shows the radial displacements ΔR in the pipe wall (x direction). The values of 0.27 mm and 0.02 mm as maximum axial and radial displacements respectively were developed. The radial displacement is in the direction outward of the pipe center and Fig. 15(c) shows the pipe position before (dashed line) and after the applying the load.

5.3 Comparison between the experimental and FEA results

Comparison between the experimental results and those obtained from the theoretical analysis by FEA for some selected cases of pipe specimens of diameter 89.1 mm and 114.3 mm are

shown in Figs. 16 and 17, respectively. In general it was found that in the elastic range of loads a good agreement between the theoretical and experimental results for all the pipe specimens were obtained.

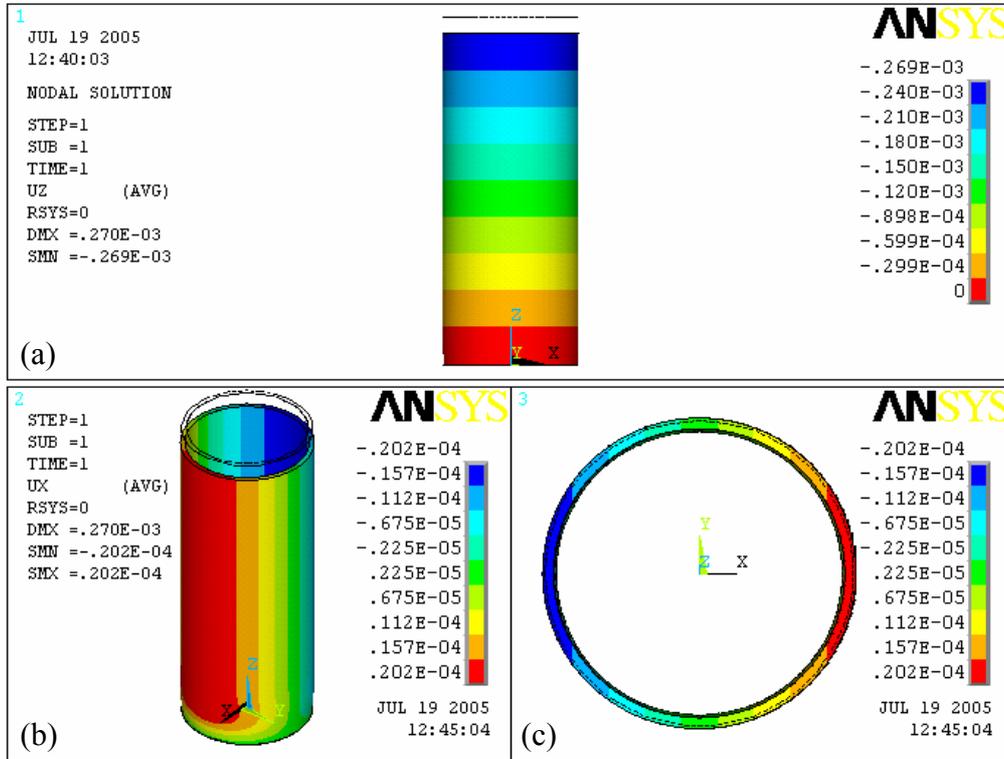


Fig. 15: Axial and radial displacements in the pipe wall

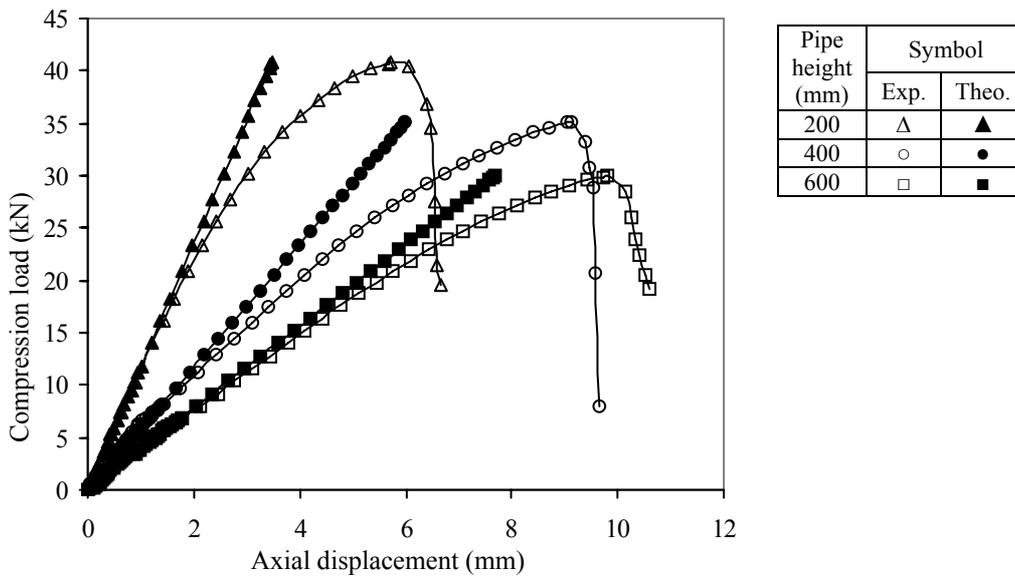


Fig. 16: Experimental and theoretical results of the 89.1 mm diameter pipe specimen tests

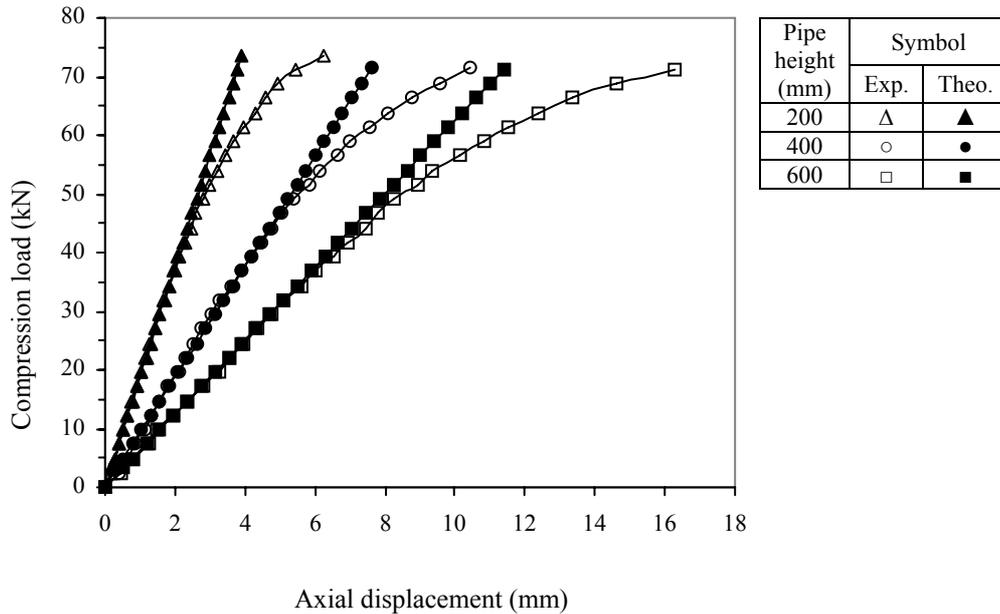


Fig. 17: Experimental and theoretical results of the 114.3 mm diameter pipe specimen tests

6. CONCLUSION

The collapse of axially loaded PVC-U pipes was investigated experimentally to evaluate the capability of the pipe to be used as a subsurface water storage, which is required to support an axial load of 3.4 kN. Short-term axial compression tests of the specimens demonstrate that, the PVC-U pipes have a capability to support axial static load of more than 3.4 kN.

In general it was found that, a reduction in the maximum load that the pipe can support occurs when the pipe height is increased and this reduction is lower with greater pipe diameter. Increasing the pipe thickness and diameter increases the pipe strength.

Euler buckling was observed in the pipe specimens with a diameter of 89.1 mm and height of 500 mm and 600 mm but it was not observed with a diameter of 114.3 mm for all the pipe specimen heights.

Finite element analysis of the pipe specimens reveal that, the stress concentrates around the bottom of specimens when applying an axial load and this gives an explanation for the reason of failure of most specimens at this location in the experimental tests. Good agreement was found between the experimental results and those obtained from the theoretical analysis by using the classical formulas and the finite element analysis.

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APPENDIX A

The formulas related to a cylindrical vessel with internal radius R , thickness t and height L and subjected to an axial tension load are,

$$\sigma_z = \frac{p}{t} \quad (\text{A.1})$$

$$\sigma_c = 0$$

$$\Delta L = \frac{pL}{Et} \quad (\text{A.2})$$

$$\Delta R = \frac{p\nu R}{Et} \quad (\text{A.3})$$

where, σ_z is the longitudinal or axial stress, σ_c the circumferential, or hoop, stress, p the uniform axial load per unit length, E the modulus of elasticity, ΔL the axial displacement, ν the poisson's ratio, and ΔR is the radial displacement.

Table A.1: Theoretical results of 89.1 mm pipe diameter

Pipe diameter (mm)	Pipe thickness (mm)	Pipe height (mm)	R/t	Classical formula			ANSYS FEA results		
				Stress (kN/m ²)	Vertical displacement (mm)	Radial displacement (mm)	Max. Stress (kN/m ²)	Vertical displacement (mm)	Radial displacement (mm)
89.1	3.2	100	12.92	3919	0.15	0.0225	5497	0.144	0.0254
		200	12.92	3919	0.29	0.0225	5497	0.288	0.0254
		300	12.92	3919	0.44	0.0225	5497	0.433	0.0254
		400	12.92	3919	0.58	0.0225	5497	0.578	0.0254
		500	12.92	3919	0.73	0.0225	5497	0.723	0.0254
		600	12.92	3919	0.87	0.0225	5497	0.868	0.0254
	3.8	100	10.72	3323	0.12	0.0188	4708	0.118	0.0215
		200	10.72	3323	0.25	0.0188	4708	0.248	0.0215
		300	10.72	3323	0.37	0.0188	4708	0.366	0.0215
		400	10.72	3323	0.49	0.0188	4708	0.487	0.0215
		500	10.72	3323	0.62	0.0188	4708	0.619	0.0215
		600	10.72	3323	0.74	0.0188	4708	0.733	0.0215
	5.0	100	7.91	2562	0.09	0.0141	3680	0.089	0.0165
		200	7.91	2562	0.19	0.0141	3680	0.19	0.0165
		300	7.91	2562	0.28	0.0141	3680	0.282	0.0165
		400	7.91	2562	0.38	0.0141	3680	0.382	0.0165
		500	7.91	2562	0.47	0.0141	3680	0.473	0.0165
		600	7.91	2562	0.57	0.0141	3680	0.571	0.0165
6.1	100	6.30	2128	0.08	0.0114	3082	0.0768	0.0137	
	200	6.30	2128	0.16	0.0114	3082	0.156	0.0137	
	300	6.30	2128	0.24	0.0114	3082	0.234	0.0137	
	400	6.30	2128	0.32	0.0114	3082	0.313	0.0137	
	500	6.30	2128	0.39	0.0114	3082	0.392	0.0137	
	600	6.30	2128	0.47	0.0114	3082	0.471	0.0137	

Table A.2: Theoretical results of 114.3 mm pipe diameter

Pipe diameter (mm)	Pipe thickness (mm)	Pipe height (mm)	R/t	Classical formula			ANSYS FEA results		
				Stress (kN/m ²)	Vertical displacement (mm)	Radial displacement (mm)	Max. Stress (kN/m ²)	Vertical displacement (mm)	Radial displacement (mm)
114.3	4.0	100	13.29	2442	0.09	0.018	3532	0.090	0.0202
		200	13.29	2442	0.18	0.018	3532	0.180	0.0202
		300	13.29	2442	0.27	0.018	3532	0.270	0.0202
		400	13.29	2442	0.36	0.018	3532	0.360	0.0202
		500	13.29	2442	0.45	0.018	3532	0.450	0.0202
		600	13.29	2442	0.54	0.018	3532	0.541	0.0202
	4.7	100	11.16	2091	0.08	0.015	3049	0.081	0.0173
		200	11.16	2091	0.15	0.015	3049	0.152	0.0173
		300	11.16	2091	0.23	0.015	3049	0.230	0.0173
		400	11.16	2091	0.31	0.015	3049	0.313	0.0173
		500	11.16	2091	0.39	0.015	3049	0.389	0.0173
		600	11.16	2091	0.46	0.015	3049	0.461	0.0173
	6.4	100	7.93	1560	0.06	0.011	2302	0.062	0.0129
		200	7.93	1560	0.12	0.011	2302	0.120	0.0129
		300	7.93	1560	0.17	0.011	2302	0.172	0.0129
		400	7.93	1560	0.23	0.011	2302	0.233	0.0129
		500	7.93	1560	0.29	0.011	2302	0.292	0.0129
		600	7.93	1560	0.35	0.011	2302	0.347	0.0129
9.5	100	5.02	1082	0.04	0.007	1650	0.0388	0.0089	
	200	5.02	1082	0.08	0.007	1650	0.0810	0.0089	
	300	5.02	1082	0.12	0.007	1650	0.123	0.0089	
	400	5.02	1082	0.16	0.007	1650	0.161	0.0089	
	500	5.02	1082	0.20	0.007	1650	0.202	0.0089	
	600	5.02	1082	0.24	0.007	1650	0.239	0.0089	